

Genetically Optimised Design Of Irrigation System Network Using Epanet As A Tool

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ABSTRACT

An irrigation system network design is just like a water distribution system that parcels water from the source reservoir to the respective destination nodes. The aim of this paper is to propose such a system that transmits water by finding the optimal pipe diameters, so that the overall network cost is minimised. For this, we use Genetic Algorithm, coupling it with a network solving tool called Epanet. Also, Rank selection is used to select the best members from the population that will form input to the next generation. Penalty cost is an impacting factor in Cost calculation. The main aim of this study is to reduce the penalty cost of the network as much as possible but at the same time our network cost should not be increased so much. So, in all GA provides an optimum solution after certain number of iterations.

Keywords:

Irrigation network, Genetic Algorithm, Epanet, optimum, Rank Selection, Fitness Function, Network cost, penalty cost.

1. INTRODUCTION

An irrigation system is a water distribution network that transmits water from the source reservoir to the consumers' locations. It consists of elements such as pipes, valves, pumps, tanks and reservoirs. The most important consideration in designing and operating an irrigation system is to satisfy consumer demands under a range of quantity and quality considerations during the entire lifetime for the expected loading conditions.

2. FORMULAE USED

The aim of the irrigation system network design is to find the optimal pipe diameter for each pipe in the network for a given layout, demand loading conditions, and an operation policy. Our model selects the optimal pipe sizes in the final network satisfying all implicit constraints (e.g. conservations of mass and energy), and explicit constraints (e.g. pressure head and design constraints). If the hydraulic head constraint is violated, the penalty cost is added to the network cost. However, diameter constraints enforce the evolutionary algorithms to select the trial solution within a predefined limit. The hydraulic information obtained from network solver is then passed to the GA for the computation of fitness of

the design. The fitness of a trial solution representing a pipe network design is based on the hydraulic performance of the network. It consists of two parts: (1) network cost and (2) penalty cost.

The network cost is calculated as the sum of the pipe costs where pipe costs are expressed in terms of cost per unit length. Total network cost is computed as follows:

$$C = \sum_{K=1}^N c_k(D_k) L_k$$

where $c_k(D_k)$ = cost per unit length of the kth pipe with diameter D_k , L_k = length of the kth pipe, and N = total number of pipes in the system.

The penalty cost is based on the degree of pressure head violation. The penalty functions may be defined, as

$$C1 = P * C_{max} * \text{Max} (H_{min} - H_i)$$

if the pressure is less than minimum limit and greater than zero; or

$$C1 = 2 * P * C_{max} - 2 * C$$

if the pressure is less than or equal to zero.

In above two equations, P is the penalty cost coefficient, C_{max} is the maximum possible cost that is calculated based on the largest commercial pipe available, $(H_{min} - H_i)$ is the maximum pressure deficit, and C is the actual network cost. The maximum pressure deficit is the difference between the required head (H_{min}) at each node and the head found after simulation (H_i). If the pressure head is greater than the minimum required limit, no extra cost is charged to the network cost.

The mathematical formulation of water distribution network can thus be stated as follows:

Minimize Cost $C =$ Network cost + Penalty cost

Subjected to:

$G(H,D) = 0$, a conservation of mass and energy equation

$H_i \geq H_{i: min}$ nodal pressure head bounds

$D_{min} < D(k) < D_{max}$, constraints related to design parameters

where,

$H_i =$ pressure head at node i ,

$H_{i: min} =$ minimum head required at the same node,

$D(k) =$ decision variables (pipe sizes).

3. SOLVING THE PROBLEM THROUGH GENETIC ALGORITHM

Here we attempt to solve the problem using genetic algorithms (GAs), an evolutionary optimization technique. GAs is founded as a randomized search and optimization technique based on Darwin's theory of the "survival of the fittest" and a stochastic information exchange procedure.

3.1. STEPS FOR THE ALGORITHM

3.1.1. [Start] Generate random population of n chromosomes (suitable solutions for the problem).

3.1.2. [Fitness] Evaluate the fitness $f(x)$ of each chromosome x in the population.

3.1.3. [New population] Create a new population by repeating following steps until the new population is complete.

3.1.4. [Selection] Select two parent chromosomes from a population according to their fitness (the better fitness, the bigger chance to be selected).

3.1.5. [Crossover] With a crossover probability cross over the parents to form a new offspring (children). If no crossover was performed, offspring is an exact copy of parents.

3.1.6. [Mutation] With a mutation probability mutate new offspring at each locus (position in chromosome).

3.1.7. [Accepting] Place new offspring in a new population.

3.1.8. [Replace] Use new generated population for a further run of algorithm.

3.1.9. [Test] If the end condition is satisfied, **stop**, and return the best solution in current population.

3.1.10. [Loop] Go to step 2

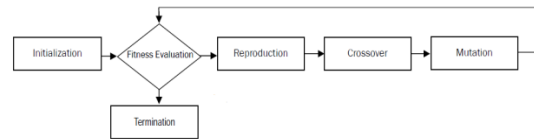


Fig. 1: GA Procedure

4. OVERALL PROCEDURE FOLLOWED

4.1 Generate N population of points randomly in the solution space: These randomly generated chromosomes will serve as a pipe diameter for EPANET network. These pipe diameters are then supply to EPANET to calculate cost and pressure deficit.

```

enter size of population15
enter size of chromosome5

randomly generated chromosome values is:
10 22 14 18 20
12 22 12 14 4
2 16 18 16 24
2 20 10 1 14
3 4 12 12 24
24 14 24 6 22
4 8 20 2 12
6 12 8 8 10
22 8 16 16 20
3 4 18 3 1
6 10 16 24 4
16 24 3 14 4
22 18 2 24 18
2 3 22 22 24
22 6 12 20 1

enter the no. of nodes in a network
    
```

Fig 2: Random population generation

4.2 Compute the network cost for each of the N from the randomly pipe sizes available in the market: In this step program calculate the network cost. Network cost is the function of the length of pipe and the cost per unit diameter. Cost per unit diameter is taken from the commercially available pipe diameters.

The formula for calculating cost is:

$$\text{Network Cost} = \sum_{k=1}^N c_k(D_k)L_k$$

where $c_k(D_k)$ = cost per unit length of the kth pipe with diameter D_k , L_k = length of the kth pipe, and N = total number of pipes in the system.

```
randomly generated chromosome values is:
10 22 14 18 20
12 22 12 14 4
2 16 18 16 24
2 20 10 1 14
3 4 12 12 24
24 14 24 6 22
4 8 20 2 12
6 12 8 8 10
22 8 16 16 20
8 4 18 3 1
6 10 16 24 4
16 24 3 14 4
22 18 2 24 18
2 3 22 22 24
22 6 12 20 1

enter the no. of nodes in a network4
enter the values of length of pipe1000 1000 1000 1000 1000
value of cmax is2750000
Network cost is:692000
enter the value of pressure for nodes
```

Fig 3: Calculation of Network Cost

4.3 Perform hydraulic analysis of each network:

The randomly generated population is serving as pipe diameter that is used in the network. Run EPANET and measure nodal pressure for each set of diameter. This pressure deficit is then use to calculate the penalty cost.

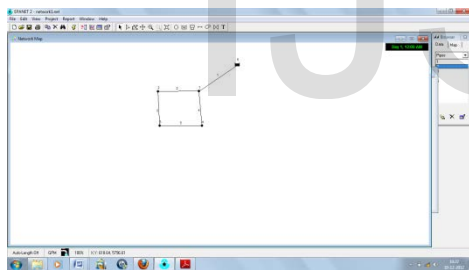


Fig. 4 : EPANET network

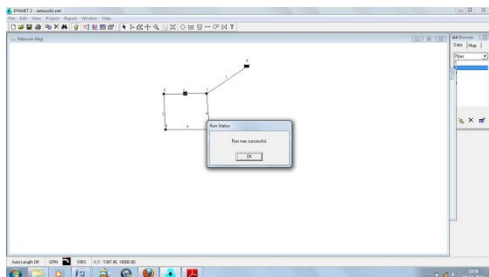


Fig. 5 : Execution of EPANET

Fig.6: Result table of Pressure at every node

4.4 Compute penalty cost, if the nodal head at any node is less than the required minimum: In this step program calculate the penalty cost. Penalty cost depends upon the nodal pressure deficit. Penalty cost equation is:

If the pressure is less than minimum limit and greater than zero

$$C1 = P * C_{max} * \text{Max} (H_{min} - H_i)$$

if the pressure is less than or equal to zero.

$$C1 = 2 * P * C_{max} - 2 * C$$

Where P is the penalty cost coefficient, C_{max} is the maximum possible cost that is calculated based on the largest commercial pipe available, $(H_{min} - H_i)$ is the maximum pressure deficit, and C is the actual network cost. The maximum pressure deficit is the difference between the required head (H_{min}) at each node and the head found after simulation (H_i). If the pressure head is greater than the minimum required limit, no extra cost is charged to the network cost.

```
24 14 24 6 22
4 8 20 2 12
6 12 8 8 10
22 8 16 16 20
8 4 18 3 1
6 10 16 24 4
16 24 3 14 4
22 18 2 24 18
2 3 22 22 24
22 6 12 20 1

enter the no. of nodes in a network4
enter the values of length of pipe1000 1000 1000 1000 1000
value of cmax is2750000
Network cost is:692000
enter the value of pressure for nodes63.94 78.15 63.24 298.50
penalty cost is:0
total cost & fitness is:692000
Network cost is:471000
enter the value of pressure for nodes
```

Fig. 7: Penalty cost calculation

4.5 Calculate the total cost of the network: The total cost of the network is the sum of the network cost and the penalty cost found in steps 2 and 4 respectively.

$$\text{Total Cost } C = \text{Network cost} + \text{Penalty cost}$$

4.6 The total cost found in step 4.5 is used as the fitness value for each of the trial network: this fitness function will further help in selection procedure, where two chromosomes are selected from the randomly generated population.

4.7 Two Chromosomes are selected from the population based on their fitness value and apply GA: In this step Rank selection is used in order to select the two chromosomes from the randomly generated population. The two selected chromosome will serve as our parent chromosome and further take part in crossover and mutation in order to generate child chromosome.

Rank selection first ranks the population and then every chromosome receives fitness from this ranking. The worst will have fitness 1, second worst 2 etc. and the best will have fitness N.

```

enter the value of pressure for nodes-23.33 -9.12 -24.02 211.23
penalty cost is:-6911999.996865
total cost & fitness is:-5748999.996865

Network cost is:538000
enter the value of pressure for nodes63.97 77.94 63.03 298.28
penalty cost is:0
total cost & fitness is:538000

selected location is5
selected cost is
174000
population is
8 4 18 3 1
selected location is7
selected cost is
471000
population is
12 22 12 14 4
Two Point Crossover
Two Point Crossover
Enter crossover point1 :_
    
```

Fig.8: Rank Selection

4.8 Corresponding Crossover and Mutation is performed on the chromosomes: In this step crossover and mutation is performed on the parent chromosome found in step 7. Crossover operator is to generate child chromosomes from two 'parent' chromosomes by combining the information extracted from the parents. Mutation is responsible for the genetic diversity of the new chromosome.

```

selected location is7
selected cost is
471000
population is
12 22 12 14 4
Two Point Crossover
Two Point Crossover
Enter crossover point1 :2
Enter crossover point2 :5

Parent1 : 8 4 18 3 1
Parent2 : 12 22 12 14 4
Child1 : 8 22 12 14 1
Child2 : 12 4 18 3 4

Interchanging Mutation:
Enter Mutation sites 1:
    
```

Fig. 9 : Crossover

```

Enter crossover point1 :2
Enter crossover point2 :5

Parent1 : 8 4 18 3 1
Parent2 : 12 22 12 14 4
Child1 : 8 22 12 14 1
Child2 : 12 4 18 3 4

Interchanging Mutation:
Enter Mutation sites 1:
3
Enter Mutation site 2:
4

After Mutation
8 22 14 12 1
Network cost is:435000
enter the value of pressure for nodes
    
```

Fig 10: Mutation

4.9. Cost calculation for newly generated children: In this step program calculate total cost (network cost + penalty cost) using two newly generated child chromosomes as pipe diameters of network. If this cost is optimal then we stop else we evolve new population.

```

DOSBox 0.74, Cpu speed: max 100% cycles, Frameskip 0, Program: TC
2
Enter Mutation site 2:
5

After Mutation
22 12 20 12 12 1 8 2
Network cost is:658000
enter the value of pressure for nodes 78 76 89 76 65 69

penalty cost is:0
total cost & fitness of child chromosome is:658000

Enter Mutation sites 1:
5
Enter Mutation site 2:
5

After Mutation
12 6 10 1 16 24 0 12
Network cost is:911000
enter the value of pressure for nodes 67 65 87 76 64 72

penalty cost is:0
total cost & fitness of child chromosome is:911000
optimum solution obtained?
    
```

Fig 11: Total cost Calculation

4.10 If the cost is optimized then STOPS: If calculated cost is optimum then stop and the respective child chromosome is our solution else go to step 11, else generate new population and go to step 5.7.

```

DOSBox 0.74, Cpu speed: max 100% cycles, Frameskip 0, Program: TC
Enter Mutation sites 1:
3
Enter Mutation site 2:
6
After Mutation
12 6 18 1 16 24 8 12
Network cost is:911000
enter the value of pressure for nodes 67 65 87 76 64 72
penalty cost is:0
total cost & fitness of child chromosome is:911000
optimum solution obtained?n
14 3 12 20 22 2 6 3
14 1 4 24 1 18 2 16
12 6 18 1 16 24 8 12
22 12 20 12 12 1 8 2
22 12 24 1 16 1 8 2
10 20 20 20 6 10 10 2
1 1 16 22 1 20 14 1
12 24 22 6 22 3 1 24
12 3 18 16 10 20 14 14
12 6 20 12 12 18 8 12
enter the no. of nodes in a network
    
```

Fig. 12: Evolution of new population

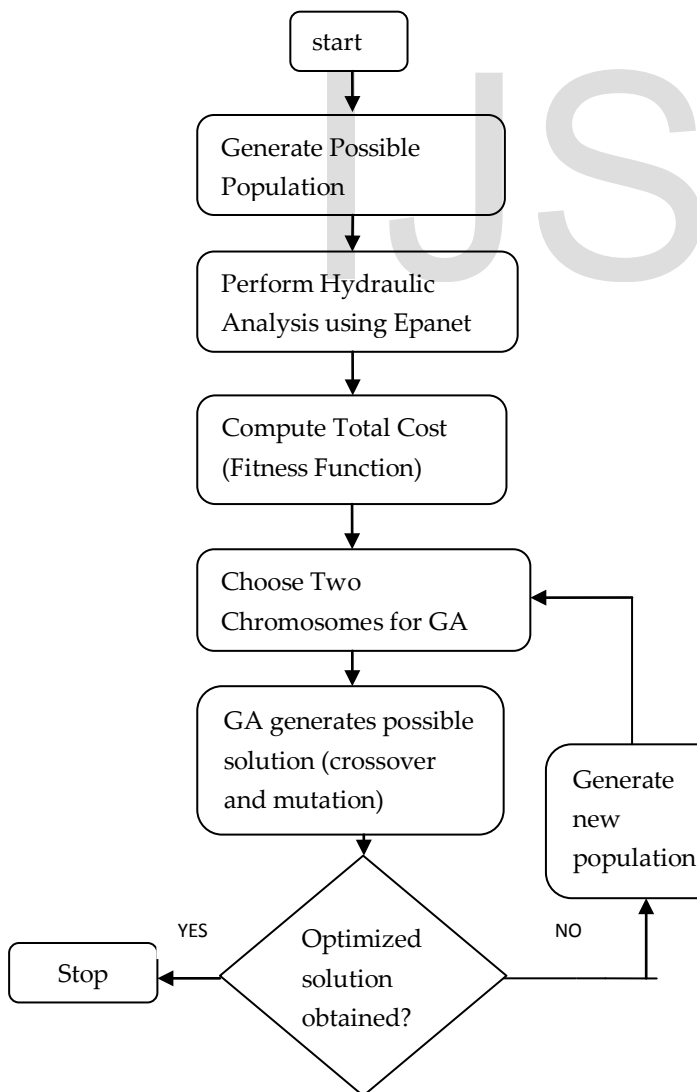


Fig 13: Optimization Flowchart of Irrigation Network

5. RESULT AND ANALYSIS

Genetic Algorithm is applied in conjunction with EPANET to find optimum results. For cost per unit diameter it uses commercially available pipe sizes.

TABLE 1- COST DATA FOR PIPES

Diameter (In)	Diameter(Mm)	Cost
1	25.4	2
2	50.8	5
3	76.2	8
4	101.6	11
6	152.4	16
8	203.2	23
10	254.0	32
12	304.8	50
14	355.6	60
16	406.4	90
18	457.2	130
20	508.0	170
22	558.8	300
24	609.6	550

In this case a simple network optimization is performed. The network has 4 node, one reservoir (water supply mean) and 5 pipes.

The following GA parameters were established for the problem.

Population= 30
 Generation= 50

Rank Selection

Two point crossover

Interchanging mutation

Following are the EPANET parameters :

Minimum pressure requirements= 60.00 psi

Cmax=2750000

Penalty coefficient=0.004

TABLE 2- PIPE PROPERTY OF EACH PIPE IN SAMPLE NETWORK

Pipe Id	Length(M)	Roughness
1	1000	130
2	1000	130
3	1000	130
4	1000	130
5	1000	130

TABLE 3- DEMAND AND ELEVATION AT EACH NODE

Node	Demand(GPM)	Elevation(Feet)
Node 1	22.71	541.33
Node 2	27.25	508.53
Node 3	45.42	542.93
Node 4	74.95	492.12
Reservoir	N/A	688.97

Data Input: data is to be inputted to the EPANET. It contains the possible set of diameters generated by GA. Then calculate result that is the pressure at each node.

TABLE 4- RANDOMLY TAKEN PIPE DIAMETERS

N O.	Pipe1 Diamet er(in)	Pipe2 Diamet er(in)	Pipe3 Diamet er(in)	Pipe4 Diamet er(in)	Pipe5 Diamet er(in)
1	10	22	14	18	20
2	12	22	12	14	4
3	2	16	18	16	24
4	2	20	10	1	14

5	3	4	12	12	24
6	24	14	24	6	22
7	4	8	20	2	12
8	6	12	8	8	10
9	22	8	16	16	20
10	8	4	18	3	1
11	6	10	16	24	4
12	16	24	3	14	4
13	22	18	2	24	18
14	2	13	22	22	24
15	22	6	12	20	1

Calculate pressure at each node for all possible set of diameters, this pressure helps in calculating penalty cost:

TABLE 5- PRESSURE AT EACH NODE FOR EACH TRAIL IN SAMPLE N/W

No.	Node 1 Press ure(p si)	Node2 Pressu re(psi)	Node3 Pressu re(psi)	Node4 Pressu re(psi)	Total Cost(Rs) (N/W cost+Penalt y cost)
1	63.94	78.15	63.24	298.50	692000
2	63.96	78.17	63.26	298.51	471000
3	-23.33	-9.12	-24.02	211.23	865000+(-5123999)=-4258999
4	-23.33	-9.12	-24.03	211.23	269000+(-1547999)=-1278999
5	51.86	66.06	51.16	286.42	669000+(186780)=855780
6	63.97	78.18	63.28	298.53	1476000

7	60.99	75.14	60.24	295.55	259000
8	63.56	77.77	62.85	298.11	144000
9	63.97	78.18	63.28	298.53	673000
10	63.87	76.36	61.46	296.72	174000
11	63.56	77.75	62.85	298.10	699000
12	63.97	78.18	62.90	298.16	719000
13	63.97	78.18	63.28	298.53	1115000
14	-23.33	-9.12	-24.02	211.23	1163000+(-6911999)=-5748999
15	63.97	77.94	63.03	298.28	538000

The result is obtained after applying selection, crossover and mutation on the randomly generated diameters. The optimum result is obtained after applying 5 iterations. Our optimum result is minimum cost of the network.

Table 6 Result after applying GA

Diameters	Pipe 1	Pipe 2	Pipe 3	Pipe 4	Pipe 5	Total Cost
Child 1	8	22	14	12	1	435000(not optimum)
Child 2	12	18	4	3	4	210000(optimum)

Optimum result will serve as the final diameter that is to be applied in the EPANET. This has been noticed that GA give result after 5 iteration. Minimum cost is obtained with Zero penalty cost.

7. CONCLUSION

Optimal water distribution network design is a complex task. Various search algorithms have been proposed and attempted. Main concerns are to achieve the optimal solution with the minimum design cost and, at the same time, satisfies required minimum pressure head at certain demand nodes and can use only commercially available pipe sizes.

In this study Genetic Algorithm, has been coupled with the widely used irrigation network software, EPANET, and applied to irrigation network designs. It has been

observed that the program reaches to the optimum result after some iteration.

Rank selection is used to select the best members from the population that will form input to the next generation.

Penalty cost is very impacting factor in Cost calculation. The main aim of this study is to reduce the penalty cost of the network as much as possible but at the same time our network cost should not be increased so much.

The study shows that GA yields categorically better performance in term of optimal network design cost and/or computational speed. It has been seen that, traditional methods are not capable to design the network required by the time. Traditional heuristics have the limitations with the possible constraints. The proposed genetic algorithm approach can provide the good results with the required constraints.

8. REFERENCES

- [1] A. Vasan and Slobodan P. Simonovic "Optimization of Water Distribution Network Design Using Differential Evolution". Journal Of Water Resources Planning And Management © ASCE / MARCH/APRIL 2010
- [2] R. Mohamad Bagher and J. Payman "Water Delivery Optimization Program, of Jiroft Dam Irrigation Networks by Using Genetic Algorithm" Int. J. Advance. Soft Comput. Appl., Vol. 1, No. 2, November 2009 ISSN 2074-8523
- [3] Perez-Lopez and Ribas, "Efficient Scheduling Irrigation on Optimization Genetic Algorithms", *Journal of Irrigation and Drainage*, 312-327. (2007)
- [4] Zecchina, Simpsona, Maiera,Leonarda, J. Robertsb, Berrisforda "Application of two ant colony optimisation algorithms to water distribution system optimization". *Mathematical and Computer Modelling* 44 (2006) 451-468
- [5] Hossein M. V. Samani and Alireza Mottaghi, "Optimization of Water Distribution Networks Using Integer Linear Programming", *J. Hydr. Engrg.*, Volume 132, (May 2006), Issue 5, pp. 501-509.
- [6] D. Naghesh Kumar; K. Srinivasa Raju and B. Ashok, "Optimal Reservoir Operation for Irrigation of Multiple Crops Using Genetic Algorithms". *Journal of Irrigation and Drainage Engineering*, 123-129. (2006).
- [7] M. H. Afshar and M. A. Marino, "A Convergent Genetic Algorithm for Pipe Network optimization", *Scientia Iranica, Sharif University of Technology*, October 2005, pp. 392-401
- [8] Cunha , Ribeiro(2003) "Tabu search algorithms for water network optimization" *European Journal of Operational Research* 157 (2004) 746-758
- [9] Shie-Yui Liong and Md. Atiquzzaman, "Optimal design of water distribution network using shuffled complex evolution" *Journal of The Institution of Engineers, Singapore* Vol. 44 Issue 1 2004
- [10] Robin Wardlaw and Kampanad Bhaktikul, "Comparison of Genetic Algorithm and Linear Programming Approaches for

Lateral Canal Scheduling". *Journal of Irrigation and Drainage Engineering*, 311-317. (2004).

[11] Eusuff, M.M. and Lansey, K.E.,(2003). "Optimization of Water Distribution Network Design Using the Shuffled Frog Leaping Algorithm." *Journal of Water Resources Planning and Management*, ASCE, Vol. 129, No. 3, pp. 210-225

[12] Maier, H. R., et al.(2003). "Ant colony optimization for design of water distribution systems." *J. Water Resour. Plann. Manage.*, 129(3), 200– 209.

[13] R.S.V. Teegavarapu and S.P. Simonovic, Optimal Operation of Reservoir System Using Simulated Annealing, *Water Resources Management*, vol. 16, pp. 401-428, 2002

[14] Wu, Z.Y., Boulos, P.F., Orr, C.H., and Ro, J.E. "Rehabilitation of Water Distribution

Systems Using Genetic Algorithms." *Journal AWWA*, July (2001).

[15] S.F. Kuo, C.W. Liu, and G.P. Markley, Application of the Simulated Annealing Method to Agricultural Water Resources Management, *J. agric. Engng. Res.*, vol. 80, no. 1, pp. 109-124, 2001.

[16] Boulos, P.F., Wu, Z.Y., Orr, C.H., and Ro, J.J. "Least Cost Design and Rehabilitation of Water Distribution Systems Using Genetic Algorithms." *Proceedings of the AWWA IMTech Conference*, April 16-19, Seattle, WA (2000).

[17] Wu, Z. Y., Boulos, P.F., Orr, C.H., and Ro, J.J. "An Efficient Genetic Algorithm Approach to an Intelligent Decision Support System for Water Distribution Networks." *Proceeding of the 4th International HydroInformatics Conference*, July 23-27, Iowa Institute of Hydraulic Research, Iowa City, IA (2000).

[18] Cunha, M.D.C., and Sousa, J. (1999). "Water Distribution Network Design Optimization: Simulated Annealing Approach." *Journal of Water Resources Planning and Management*, Vol. 125, No. 4, pp.215-221.

[19] Gupta, I., Gupta, A., and Khanna, P. (1999). "Genetic algorithm for optimization of water distribution systems." , *Environmental Modelling & software* Vol.-4, pp. 437-446.

[20] Xu, C., and Goulter, I., "Reliability-Based Optimal Design of Water Distribution Networks.", *J. Water Resour. Plng. and Mgmt.*, ASCE, 352-362, 1999.